0. EXECUTIVE SUMMARY

0.1 INTRODUCTION

The Highway starting from Jeevargi connecting Maski, Siruguppa, Bellary, Challakere, Hiriyur, Turuvekere, Shrirangapattana, Mysore, and Chamarajanagar, which previously comprised of SH was recently upgraded and declared as NH-150A by Ministry of Road Transport & Highways (MoRT&H), Government of India (GOI).

The Ministry of Road Transport & Highways (MoRT&H), Government of India (GOI), (Public Works, Ports And Inland Water Transport Department), Office of The Executive Engineer, National Highways, Chitradurga division has appointed M/s. Feedback Infra Pvt. Ltd. for providing the consultancy services for preparation of Feasibility Study, Detailed Project Report, Survey, Preparation of Land Plan for widening to Two lane Paved shoulder from Bellary to Hiriyur section in the state of Karnataka.

The Contract Agreement for the assignment was signed on 7th May 2015; the Consultancy services were commenced with effect from 7th May 2015.

The Consultant has studied the feasibility of the project and submitted the Detailed Project Report along with EPC Schedules Executive Engineer, National Highways, Chitradurga division vide letter no FIPL/Highways/DPR/NH-PWD/CTR/2016-17/485 and FIPL/Highways/DPR/NH-PWD/CTR/2016-17/486A

From the Traffic surveys and analysis, it was concluded that the project road requires four lane with paved shoulders (PCUs are more than 10000), hence the Project is transferred from Ministry of Road Transport & Highways to National highways authority of India vide Gazette notification S.O. 1096 (E) dated 23.11.2016. The Tripartite agreement for the Consultancy services was signed on 04 May 2017 and the project preparation activities commenced subsequently.

As per the Tripartite agreement Project road has been divided into three packages and Package wise Details given in below table

SI.	Daalaana	Ducinet Section	Existing km		Design Chainage		Design Length	
no	Package	Project Section	From	То	From	То	in km	
1	a done	Bellary to Byrapura (include Bellary Bypass)	253.600	309.150	253+600	308+550	54.95	
2	arei II sag	Byrapura to Challakere	309.150	359.100	308+550	358+500	49.95	
3	III	Challakere to Hiriyur (Including Challakere and Hiriyur Bypass)	359.100	413.500	358+500	414+215	55.71	

Current report depicts Final Feasibility report of first package i.e. to **Bellary to Byrapura Including Bellary Bypass**.

DESCRIPTION OF PROJECT

The State of Karnataka is located in southwest part of India. Karnataka is surrounded by Maharashtra, Goa, Andhra Pradesh, Kerala and Tamil Nadu. The NH 150A starts at Jevargi and ends at Chamarajanagar in Karnataka. The entire stretch of NH 150A passes through Gulbarga, Raichur, Bellary, Ananthpur, Chitradurga, Tumkur, Mandya, Mysore and Chamarajanagar districts and passing through important cities/towns like Jevargi, Shorapur, Lingsugur, Sindhanur, Siruguppa, Ballari, Hanagal, Challakere, Hiriyur, Huliyar, Turuvekere, Nagamangala, Pandavapura, Shrirangapattana, Mysore, Nanjanagudu and Chamarajanagar. The Total length of NH 150A is 618.62kms. The Project Road Starts at km 253.600 near Bellary and ends at km 309.150 near Byrapura. The Total Length of the project stretch is 55.55 kms. Figure 0-1 Refers to the location of the Project stretch NH 150A.

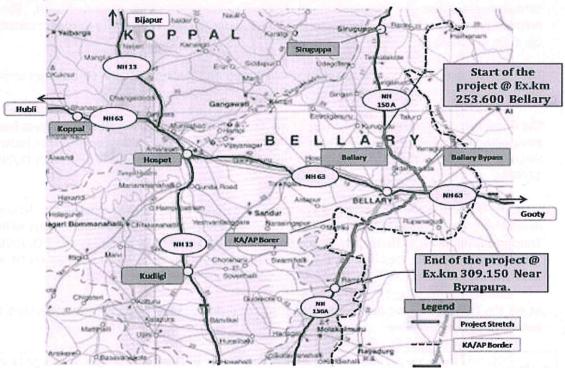


Figure 0-1: Location of Project Stretch

The project road alignment generally runs in plain terrain and passes through settlements like Bellary, Halikundhi, Obulapuram, D.Hirehal, Bommakkanahalli, Rampura, and Byrapura.

The existing alignment comprises of many sharp horizontal curves which require geometric corrections. Few among these are observed to have insufficient sight distance.

The existing road has two lane carriageways with 1 m to 1.5m paved shoulder. The condition of existing pavement varies from Good to fair.

There are 118 structures present in 55.55 km length of stretch. These include 1 Major bridge, 42 minor bridges, 1 ROB, 1 Causeway and 73 culverts. One level crossing is also present along the project stretch.

Right of Way (RoW) available varies from 15m to 30 m in rural stretches and 15 m to 20 m in urban and Semi-urban stretches.

The traffic on this stretch of NH 150A is of mixed type, with of passenger traffic, up to 55% and that of freight traffic up to 45%. Daily traffic ranges from 22261 PCUs to 15101 PCUs, equally distributed in both direction.

The predominant land use is agricultural (53.74% on LHS and 51.40 % on RHS) built up (34.74% on LHS and 35.10% on RHS) and barren land (11.52% on LHS and 13.50% on RHS)

There are 26 numbers of minor intersections & 9 numbers of major intersections along the project corridor with various categories of roads.

Large number of utility lines viz. electric / telephone lines, gas pipe lines & OFCs run parallel and across the route, which needs to be relocated for facilitating the widening.

0.2 Traffic Survey and Analysis

Traffic surveys and analyses were carried out in two phases for addressing various objectives and issues pertaining to widening of the project stretch. The surveys conducted include seven day volume counts, intersection, pedestrian/ cattle crossing, axle load and Origin Destination survey. The study aims at understanding existing traffic and travel characteristics on the project corridor and forecasting for project horizon year considering various constituent streams and for various scenarios. The results of analysis would form inputs for designing the pavement, developing capacity augmentation proposals, carrying out financial analysis, decisions regarding grade separators, pedestrian facilities, planning the tolling strategy, designing the toll plaza, wayside amenities along with design of intersections on the widened project road.

The volume count surveys were conducted at two locations, i.e., in July month of 2015. The annual average daily traffic at these locations is shown in table below.

PCU **Vehicles** Chainage Location ADT **AADT** ADT AADT 12070 12070 22261 km 269.290 Halakundi 22261 km 287.350 Tammenahalli 6520 6520 15101 15101

Table 0-1 Annual Average Daily Traffic along project corridor

The investment priorities are governed by traffic demand, assessed benefits and cost of project. Demand plays the important role that governs which type of facility / infrastructure to be created. This in turn determines likely benefits and costs to develop the same. A highway project of this nature calls for significant investment. Prediction of traffic demand becomes an important task and has to be carried out near accurately. For the design of pavement and to plan for the future maintenance programme as well as capacity augmentation and for financial evaluation, it is necessary to have realistic estimate of the size of traffic in horizon year.

Traffic forecasting is done in two ways, one with CAGR of 5 % as proposed in the ToR and the other with growth rates established by using elasticity method. This forecasting is made by determining the past trend of traffic flow along the corridor and by use of economic models developed to corelate past vehicle registration data and economic indices such as per capita income (PCI), net state domestic product (NSDP) and gross domestic product (GDP). By using the elasticity values obtained from the econometric models and the likely rate of growth of indices, population and regional influences, the mode wise growth rates are established.

Classified direction wise turning movement survey has been conducted at 2 major junctions shown in Table below along the project stretch. Type of existing intersection and structures proposed based on the turning movement survey are also shown in Table below.

Table 0-2 Turning movement survey locations

S No.	No. Chainage Location		Total Volume (PCU-12 hrs)	Peak Hour Volume (PCU)	Peak Hour
1	km 269.290	Bellary	22158	2199	18.00 – 19.00
2	km 287.350	Bellary	19913	1959	17.00 - 18.00

0.3 Axle load survey

Axle load survey has been conducted at 1locations using portable electronic weigh pads in order to plot actual loading spectrum of commercial vehicles, which create potential damage to pavement.

This damage to pavement normally qualified by a factor termed as vehicle Damage Factor (VDF). The VDF for project stretch is worked out using equivalency factors and damaging power of different axle, IRC: 37-2012. The VDF values adopted for MSA calculation are shown in Table below.

The Axle load surveys were conducted at one location, i.e., in December month of 2015. The annual average daily traffic at these locations is shown in table below.

Table 0-3 VDF values, observed and recommended

Location	Existing NH 150A km 287.350			
Mode	Bellary – Byrapura	Byrapura - Bellary		
LCV	0.79	0.15		
2-axle	3.75	2.86		
3-axle	4.86	2.31		
MAV	8.52	2.38		
Bus	1.22	1.28		

The cumulative million standard axles at count locations have been calculated for various horizon years and shown in Table below. These values are used in new pavement design.

0.4 Intersections

There are 9 major & 26 minor intersections along the project stretch. These include 9 four legged Intersections and rest are three legged Intersections. The minor & major intersections, with state highways and district roads are given in below table.

Table 0-4 List of Minor Junctions

Sl. No	Existing	Type of	Le	ads
31. IVO	km	intersection	LHS	RHS
1	258.350	3 Legged	Ashok Nagar	undindakten mesta irlan
2	258.665	4 Legged	Gayatri Nagar	To Shanti Nagar
3	260.340	3 Legged	accomplishing dark.	To Shivalinga Nagar
4	260.720	3 Legged	erres dan artic pead	Devi Nagar Circle
5	262.960	3 Legged	To Bangalore Road	ntram internation is
6	264.020	3 Legged	elikanga aktili sa	To Cowl Bazar
7	264.750	4 Legged	To Phase I	To Guggrahatti
8	268.820	3 Legged	Halakundhi	
9	273.820	3 Legged	Hiredahalu	and the second second
10	275.800	3 Legged		Obulapuram

CL N-	Existing	Type of	Le	ads
SI. No	km	intersection	LHS	RHS
11	278.724	3 Legged		D HireHal
12	278.960	3 Legged		D.Hirehal
13	281.060	3 Legged		Madenahalli
14	281.600	3 Legged	Jajarakal	and the same
15	282.725	3 Legged	n lungare et a la company	To Lingamanahalli
16	284.930	4 Legged	To Kanakuppe	To Bommaghatta
17	286.584	4 Legged	To Tammenahalli	Bommaghatta
18	287.955	3 Legged		To Rajapura
19	289.172	4 Legged	Sirekola	Konapura
20	293.865	3 Legged		Rampura Village
21	294.160	3 Legged		Rampura Village
22	294.420	3 Legged	a detada adina all	To Pakkurthy
23	300.590	3 Legged	To Siddapura	alphar from the control of
24	301.830	4 Legged	Gowrasamudra	Chikkanahalli
25	303.840	3 Legged	To Nagasamudra	Norman annual air Airlin
26	308.495	3 Legged	To Nagasamudra	

Table 0-5 List of Major Junctions

CL N	Estation Inc.	Type of	Lead	s To	Remarks
SI. No	Existing km	Intersection	Left	Right	Kemarks
1 -	260.020	4 Legged	Moka	Hospet Road	NH-63
2	261.000	3 Legged	AND A LEADING	Fort Main Road	a server of poly
3	261.235	3 Legged	Allen and T	Hospet Road	Matha Road
4	261.240	3 Legged	Anantapur	2150	Bellary Moti circle
5	261.520	3 Legged		Old Trunk Road	SH-132
6	266.140	3 Legged	Kakkabevinahalli	.54 J?	NH-63
7	266.840	4 Legged	Burrayanakanahal	Kudatini	NH-63
8	294.420	3 Legged		Melinakannave	SH-131
9	296.415	4 Legged	Devasamudra	Mahadevarapura	SH-131

0.5 Engineering Surveys and Investigations

The general engineering studies conducted along the project stretch include topographic survey, Pavement composition, pavement condition surveys and pavement structural strength.

Topographic Survey

The Topographic Survey for improvement of project road was carried out by GPS, electronic Total Station and Auto level equipments. The detailed survey methodology and specifications followed are as described below.

Initially control points and traverse stations were established using GPS and Total Station equipments. Then using Auto level equipment, Double Tertiary (DT) levelling was carried out to fix temporary benchmarks with respect to GTS Benchmarks available in the area to establish the

vertical control to all TBMs and traverse stations. The detailed survey was then carried out and the data was processed for engineering design.

The following specifications were adopted for carrying out the topographic surveys. Traverse points positioning was with accuracy of 1:10,000. All traverse points were marked on ground by punching nails in to surface. TBMs are fixed on BM reference pillars fixed 250 m apart, at top of the kilometre stones, parapets of culverts and on plinth top of the buildings. Double tertiary methods have been used in levelling, taking the sum and average of three wire readings to determine reduced levels.

Cross-sections of roads are taken at an interval of 50 m. Levels were taken at the centre and edges of carriageway, edge of shoulders and levels up to property lines on both sides. Details of drain widths and depths also collected. For all the existing culverts, top levels / road levels and bed levels / sill levels were collected.

Road Inventory

The existing pavement for the entire stretch is of bituminous surface except at major bridge locations. The pavement width is predominantly two lane carriageway with 1.0 m Paved shoulders on either side and 4 lane divided carriageway exists at some built up stretches of the highway and is varies from fair to poor condition.

Pavement Condition

The survey on general pavement condition was primarily a visual exercise undertaken by means of slow drive-over survey, and supplemented with measurements wherever necessary. Visual assessment was carried out at suitable intervals at 200 m and wherever necessary, depending on variations in pavement conditions.

The existing pavement condition based on visual observation varies from good to poor. It has been observed that in most of the stretches heaving / settlements / distress were observed. This may be due to weak subgrade, improper compaction or movement of heavy loaded trucks. Summary of pavement condition is given in Table below.

Table 0-6 Summary of Pavement Condition

SI. No.	Summary	Length (km)	%
1	Good	29.35	58.00
2	Fair	23.80	47.04
3	Poor	2.40	4.74
To	tal	55.55	100

Pavement Structural Strength

Benkelman Beam deflection studies were carried out for evaluating the residual strength of the existing pavement and assessing the strengthening requirements for the existing pavement.

BBD tests have been conducted for every 3 km interval. The pavement deflection has been observed for homogeneous sections based on pavement condition survey.

The summary of Characteristic deflection is given in Table below.

Table 0-7 Summary of Characteristic Deflection

Cu No	Existin	ng Km	Lawath Kan	Characteristic	
Sr. No	From	То	Length Km	Deflection Value	
1	272.400	275.400	3	0.901	
2	275.400	281.400	6	1.132	
3	281.400	284.400	3	1.502	
4	284.400	287.400	3	1.500	
5	287.400	290.400	3	1.327	
6	290.400	293.400	3	1.398	
7	293.400	296.400	3	0.915	
8	296.400	303.400	7	0.856	
9	303.400	308.400	5	1.371	

Subgrade Investigation

The laboratory investigations of sub-grade indicate that the existing subgrade varies from location to location along the road. The 4 days soaked CBR values for 65 blows of existing sub-grade varies from 7% to 11%.

Material Investigation

Potential sources of soil for construction of embankment and subgrade (for reconstruction / new carriageway) were identified on either side of project stretch.

Borrow area quarries located in 7 locations with average CBR of 11% which is found suitable for construction.

Aggregate quarries were identified in 5 locations which are found suitable for construction.

Hydrological investigations

Hydrological investigations have been carried out for the entire project road. It has been verified that majority of the cross drainage structures are hydrologically adequate to carry the discharges of the river / streams. It has been ascertained from local enquiry and from the National Highways.

0.6 PRELIMINARY DESIGN

Geometric design

The existing alignment largely runs is in plain terrain and the design speed of 80 to 100 kmph has been adopted as per IRC: SP: 84- 2014. Geometric design viz. horizontal and vertical curves are designed as per IRC 38-1988 & IRC: SP: 23 - 1983.

Alignment proposal

After carrying out field investigations and reconnaissance survey of existing alignment, the consultants have been arrived at alignment proposals.

Widening proposals have been prepared based on availability of additional land / existing RoW, horizontal geometrics, study of existing bridges and considering road safety parameter measures. The lengths of adopted type of widening details (as per proposed chainage) are given in Table below.

Table 0-8 Length of adopted type of widening scheme

SI. No	Type of widening	Design Length Km
1	Concentric	0.800
2	Eccentric left/Right	15.030
3	Realignments	21.134
4 Bypass		17.986
	Total Length, Km	54.950

Proposed Lane widths

The width of basic traffic lane is taken to be 3.5 m. For proposed 4-lane the carriageway width will be 14m width with paved shoulders on both sides. In divided cross-section, the median will be of 5.0 m width in rural areas and of 2.5 m width in urban areas including 0.5 m shy off on both sides. The proposed carriageway details are presented in table below.

Table 0-9 Proposed Lane widths

SI. No	Design (Chainage	Longth m	Lane Width	Remarks
31. 140	From	То	To Length, m Lane Width		Kelliaiks
1	253+600	290+760	37160	4 Lane with Paved Shoulders	es la composició de pestá
2	290+760	291+060	300	4 Lane with Paved Shoulders + Service Road	Built Up Bommakkanahalli Village
3	291+060	308+550	17490	4 Lane with Paved Shoulders	r value y sapare ve

Typical Cross Sections

Based on the traffic considerations, geometric standards and existing site conditions, the following typical cross sectional elements are framed for project road.

Table 0-10 Details of Proposed Cross Section

Element	Width (m)	Total Width (m)
4-lane Divided Carriage way Cross Section in Rural	(C/S Type 1,1A,	1B,& 1D)
Main Carriageway	2 X 7.00	14
Median	1 x 4.00	4
Kerb Shyness	2 x 0.5	1
Paved shoulder	2 X 1.50	3
Earthen Shoulders	2 X 2.00	4
Utility corridor	2 x2.00	4
Drain, Future widening etc	2x15.0	30
Total	a transfer	60
4-lane Divided Carriage way with Service Cross Sect	tion in Rural (C/	S Type 1C)
Main Carriageway	2 X 7.00	14
Median	1 x 4.00	4
Kerb Shyness	2 x 0.5	1
Paved shoulder	2 X 1.50	3
Earthen Shoulders	2 X 2.00	4

Element	Width (m)	Total Width (m)
Utility corridor	2 x2.00	4
Service Road	2 X 7.0	14
Earthen Shoulders on either side of Service road	4 X 1.50	6
Drain, Future widening etc	2 X 5.0	10
Total		60
4-lane Divided Carriage way Cross Section in U	rban (C/S Type 2, 2	A & 2B)
Main Carriageway	2 X 7.00	14
Median	1 X 1.50	1.5
Kerb Shyness	2 x 0.5	1
Paved shoulder	2 X 2.0	4
Earthen Shoulders	2 X 2.00	4
Earthen Shoulders for service roads	2 X1.50	3
Service Road	2 X 7.5	15
Drain cum Foot Path	2 X 1.5	3
Utility Corridor/Footpath	2 x 2.00	4
Space left for future widening / Drain	2 X 5.25	10.5
Total		60
VUP approach Cross Section in rura	al (C/S Type 3)	to a degree of
Main Carriageway	2 x 10.5	21
Median	1 x 4.00	4
Median Shy off	2 x 0.5	1
Paved shoulder	2 X 2.0	4
Crash barrier with shyness	2 x 1.0	2
Slip road on both sides	2 x 7.0	14
Earthen shoulders	2 x 1.5	3
Divider between slip Road and VUP Approach	2 x 0.5	1
Utility Corridor	2 x 2.0	4
Drain & Slope	2 x 3.0	6
Total	raturile	60
VUP/Flyover approach Cross Section in U	Urban (C/S Type 3A	YES WITE
Main Carriageway	2 x 10.5	21
Median	1 X 1.50	1.5
Median Shy off	2 x 0.5	1
Paved shoulder	2 X 2.0	4
Crash barrier with shyness	2 x 1.0	2
Divider between slip Road and Fly Over Approach	2 x 4.250	8.5
Service road on both sides	2 x 7.5	15
Utility corridor	2 x 2	4
Footpath cum drain	2 x 1.5	3 3 -
Total	2 1 2 1 3	60
VOP Approach Cross Section in Urba	an (C/S Type 4)	
Main Carriageway	2 x 14.00	28
Median	1 x 4.00	4
Median Shy off	2 x 0.5	1

Element	Width (m)	Total Width (m)
Paved shoulder	2 X 1.5	3
Footpath cum Drain	2 x 1.5	3
Crash Barrier	2 x 0.5	1
Divider between slip Road and VOP Approach	2 x 0.5	1
Divider Shy Off	2 x 0.5	1
Slip road on both sides	2 x 5.5	11
Kerb Shyness	2 x 0.5	1
Footpath cum Drain	2 x 1	2
Utility Corridor	2 x 2.0	4
Total	and the fee	60
4-lane Divided Carriage way Cross Section in Hilly	Area (C/S Ty	pe 5)
Main Carriageway	2 X 7.00	14
Median	1 x 4.00	4
Kerb Shyness	2 x 0.5	1 1 1 F
Paved shoulder	2 X 1.50	3
Earthen Shoulders	2 X 2.00	4
Utility corridor	2 x2.00	4
Drain, Breast Wall, Catch water Drain and Future widening etc	2x15.0	30
Total	VGA	60
ROB Approach with Slip road Cross Section (ushud I Na salasia
Main Carriageway	2 x 12.0	24
Median	1 x 4.00	4
Median Shy off	2 x 0.5	a v o boo 1 w/a *
Main Carriage way Shy off	2 x 0.5	1
Footpath	2 X 1.5	3
Divider	2 x 0.5	<u>ia - 51 1 1 5 i</u>
Crash Barrier	4 x 0.5	2
Slip road on both sides	2 x 5.5	11
Earthen shoulders	2 x 1.5	3
Utility Corridor	2 x 2.0	4
Drain & Slope	2 x 3.0	6
Total	= = = = = = = = = = = = = = = = = = = =	60

Pavement Design

New Flexible pavement has been proposed for additional 2-lane & flexible overlay over existing 2-lane road. Rigid pavement is proposed only at toll plaza location.

New Flexible/Rigid Pavement Design

The pavement design basically aims at determining the total thickness of the pavement structure as well as thickness of individual structural components. The following assumptions are considered for the preliminary pavement design. The basic assumptions considered while designing are as follows.

As per IRC: SP: 84-2014 (for 4-Lane)

As per IRC: SP: 84-2014 flexible pavements shall be designed for a minimum design period of 15 years or operation period whichever is more. Stage construction will be permitted subject to the thicknesses of sub-base & base coarses are designed for 15 years & bituminous surface for a minimum of 10 years. Strengthening for the future traffic can be carried out by means of Bituminous Overlay. Rigid pavement shall be designed for a minimum design period of 30 years; stage construction shall not be permitted.

Flexible Pavement

The project road has been divided into two homogeneous sections, design for which are furnished below.

Table 0-11 Cumulative MSA at sections

Description	Secti (from D. Ch 284+	253+600 to	Section-II (from D. Ch 284+400 to 308+550)		
The second of th	LHS	RHS	LHS	RHS	
Design life of Base and Sub Base (in years) - For Stage Const.	15	15	15	15	
Design life of BT layer (in years) - For Stage Const.	10	10	10	10	
MSA for Base and Sub Base - For Stage Const.	96.67	40.62	75.14	32.63	
MSA for BT layer - For Stage Const.	55.97	23.58	43.46	18.92	
Design CBR	10%	10%	10%	10%	

Table 0-12 Flexible pavement composition (in mm) recommended for Main Carriageway

Sections	(from D.	ection-I Ch 253+600 to 84+400)	Section-II (from D. Ch 284+400 to 308+550		
Description	Description Bellary- Byra Byrapura Be		Bellary- Byrapura	Byrapura - Bellary	
BC	45	40	40	40	
DBM	100	90	95	80	
WMM	250	250	250	250	
GSB	200	200	200	200	
Subgrade Material	500	500	500	500	
Total	. 1095	1080	1085	1070	

The pavement composition of paved shoulders has been kept with the same specifications as those of the main carriageway.

Table 0-13: Flexible Pavement Compositions for Service road

	Servic	e/Slip Roa	d
Composition	MSA	Design CBR (%)	Thickness, mm
BC			40
DBM			50
WMM	10		250
GSB	10	10	200
Subgrade	TUT-DED		500
Total			1040

Table 0-14 Overlay Thickness (in mm) recommended

	NH-150A Existing Km Design Chainage Average	NH-150A Existing Km Design Chainage			Overley	Design Thickness				
No	From	То	From	То	Characteristic Deflection	MSA	BM (mm)	Overlay (mm)	BC (mm)	DBM (mm)
1	253.600	284.800	253600	284400	1.32	74.63	165	115.5	40	80
2	284.800	309.150	284400	308550	1.23	57.89	161.05	112.74	40	75

Rigid Pavement

The Rigid pavement is proposed at toll plaza location. The design as per the IRC: 58 - 2015 leads to the crust thickness of pavement as given in following table.

Table 0-15 Rigid Pavement Compositions (As per IRC: 58-2015)

Pavement Composition	Thickness in mm
Design Life (Yrs)	30
Design CBR (%)	10
PQC (mm)	280
DLC (mm)	150
GSB (mm)	150
Sub-grade	500
Total	1080

Truck lay-byes

Truck lay-byes are proposed at following locations and new Flexible pavements is proposed at these locations.

SI. No.	Existing Km	Design Chainage	Side	Name / Location
1	Bellary Bypass	255+200	Both	Bellary Bypass
2	290.625	290+180	Both	Near Bommakkanahalli

Rest Area

The location of the Rest area is given below

S. No	Existing km	Design Chainage	Side	Location	Area
1	Halakundhi Realignment	269+100	LHS	Halakundhi Realignment	2.5 Ha
2	301.000	300+800	RHS	Near Gowrasamudra	2.5 Ha

Bus Bay with Bus Shelters

There are 12 nos Bus Bay with Bus Shelters and 34 bus shelters are proposed along the project stretch.

Toll Plaza

Rigid Pavement is proposed for the toll plaza location, as it has longer life and can resist the wear and tear caused by the braking forces exerted by heavy vehicles.

One toll plaza is proposed with additional right-of-way, service lanes, toll booths, lighting, weigh-in-motion Weigh Bridge, automatic, semi automatic and manual toll booths, separate lanes for wide bodied vehicles etc. The detail of the proposed toll plaza is given below.

Table 0-16 Proposed Toll Plaza Location

	Toll Plaza Location			No of Toll Lanes*		
Sl. No.	Existing Chainage	Design Chainage	Location	LHS	RHS	
1	280.710	280+350	Near D. Hirehal	7+1	7+1	

^{*}As per DO No. NHAI/Chairman/Misc./2016 dated 26.12.2016

Proposal for Structures

There are 2 major bridges, 44 Minor bridges, 7 Flyovers, 4 VUP, 4 VUP grade-II (LVUP), 2 ROB and 2 Trumpet Interchange along with many other cross drainage works are proposed along the project stretch. Nos. of each type of structure along the project stretch is given in below Table. These structures are proposed for widening, rehabilitation or construction of new structure.

Table 0-17: Proposed Structures on Project Stretch

SI. No.	Type of structure	No's.
1	Major bridges	02
2	Minor bridges	44
3	Flyover	07
4	VUP	04
5	VUP Grade II-(LVUP)	04
6	Trumpet Interchange	02
7	ROB	02
8	Culverts	118

0.7 Cost Estimation

The cost estimation for the project is extremely important as the viability and implementation of a project depends on the project cost. Therefore, cost estimates have been carried out with due care. Estimation of preliminary cost, a primary pre-requisite for economic and financial evaluation, has been carried out for widening the existing NH stretches to 4-lane carriageway with paved shoulders on both sides including Reconstruction of the existing pavement, strengthening / widening of existing bridge structures, construction of new bridges, rehabilitation and reconstruction / widening of cross drainage structures, longitudinal drains, junction improvements, road furniture, bus bays, truck bays, way side amenities, toll plazas, etc. and is presented in below Table.

Table 0-18 Summary of Cost Estimate for Bellary to Byrapura section of NH-150A

BILL NO.	ABSTRACT OF COST - Bellary to Byrapura Section of NH-150A (D BILL NAME	Total Amount in Rs	Total Amount in Crores	Total Amount in %
WE'D	CIVIL CONSTRUCTION COST		The Property	
1	Site Clearance and Dismantling	2,44,15,461	2.44	0.25%
2	Earth Work	1,18,14,53,438	118.15	12.17%
3	Granular Sub-Base and Base Courses	1,53,38,42,169	153.38	15.80%
4	Bituminious Works		-	
4A	Flexible Pavement	1,44,77,63,809	144.78	14.91%
4B	Rigid Pavement			
5	CULVERTS		16.1	
5A	Box Culvert	44,75,68,840	44.76	4.61%
5B	Pipe Culvert	7,60,41,655	7.60	0.78%
6	BRIDGES			
6A	Repair & Rehabilitation of Structures	38,37,085	0.38	0.04%
6B	Major Bridges	19,76,76,138	19.77	2.04%
6C	Minor Bridges	1,26,88,83,425	126.89	13.07%
6D	VUP	35,18,67,519	35.19	3.62%
6E	PUP/LVUP	29,20,59,095	29.21	3.01%
6F	CUP	10,04,34,959	10.04	1.03%
6G	ROB	80,00,98,317	80.01	8.24%
6H	Flyover and Overpass	1,11,75,72,102	111.76	11.51%
7	Drainage, Protective Works & Other Services	20,54,34,089	20.54	2.12%
8	Junctions	10,10,58,103	10.11	1.04%
9	Traffic Signs, Road Marking and Other Appurtenances	19,05,73,423	19.06	1.96%
10	Miscellaneous	25,70,73,325	25.71	2.65%
11	Toll Plaza Construction @ Ch280+300	10,98,07,651	10.98	1.13%
Α	Total Civil Cost (YR: 2016-2017) =	9,70,74,60,603	970.75	
	Civil Cost per Km (Length of Project Highway-54.950 km)	17.67		
VIII	Centage (As per ministry circular dated 10th Aug 2016, for HAM project 15%	1,45,61,19,090	145.61	
IX	TOTAL PROJECT COST (IV+VIII)	11,16,35,79,694	1,116.36	
	Total Project Cost per Km	20.32		
	PRE CONSTRUCTION ACTIVITY			
	LA, R&R and Social	2,32,15,90,100	232.16	
	Environment Cost	2,62,99,369	2.63	
X	Shifting of Electrical Poles/Lines (1% of Civil Cost) =	9,70,74,606	9.71	
	Shifting of Water Supply Pipe Lines (1% of Civil Cost) =	9,70,74,606	9.71	
_	Total cost of preconstruction activities D=(a+b+c+d+e) =	2,54,20,38,681	254.20	
XI	Total Capital Cost (C+D) =	13,70,56,18,375	1,370.56	
	Total Capital Cost Per Km in Crores =	24.94		

CONCLUSIONS AND RECOMENDATION

ECONOMIC ANALYSIS

The economic internal rate obtained was return of 14.8% for development of the project road with flexible pavement, which is more than the presented 12% of discounted rate and larger positive value of NPV of net benefits indicate the firm viability of project for the proposed improvement of four laning with paved shoulders and hence recommended for implementation.

FINANCIAL ANALYSIS

It is concluded that the project is not viable on commercial BOT (Toll) format with 40 % grant for concession period of 10 and 15 years. At concession period of 20 Years, equity IRR achieves benchmark of 15% at 33.10% grant but project IRR stands much below the benchmark of 12%. However, as concession period is increased to 25 years, the equity IRR benchmark is met at 22.20% grant while project IRR is marginally short of achieving the benchmark of 12% at this level of grant. However as per Policy Matter: Technical (161/2014) Lt No.11041/218/2007-Admn Dated: 24.07.2014 (Given in Appendix 12) the project has been recommended both on EPC mode or HAM Mode.

National Highways Authority of India PNJ, Hospet

0.

EXECUTIVE SUMMARY

0.1 Introduction

The Highway starting from Jeevargi connecting Maski, Siruguppa, Bellary, Challakere, Hiriyur, Turuvekere, Shrirangapattana, Mysore, and Chamarajanagar, which previously comprised of SH was recently upgraded and declared as NH-150A by Ministry of Road Transport & Highways (MoRT&H), Government of India (GOI).

The Ministry of Road Transport & Highways (MoRT&H), Government of India (GOI), (Public Works, Ports And Inland Water Transport Department), Office of The Executive Engineer, National Highways, Chitradurga division has appointed M/s. Feedback Infra Pvt. Ltd. for providing the consultancy services for preparation of Feasibility Study, Detailed Project Report, Survey, Preparation of Land Plan for widening to Two lane Paved shoulder from Bellary to Hiriyur section in the state of Karnataka.

The Contract Agreement for the assignment was signed on 7th May 2015; the Consultancy services were commenced with effect from 7th May 2015.

The Consultant has studied the feasibility of the project and submitted the Detailed Project Report along with EPC Schedules Executive Engineer, National Highways, Chitradurga division vide letter no FIPL/Highways/DPR/NH-PWD/CTR/2016-17/485 and FIPL/Highways/DPR/NH-PWD/CTR/2016-17/486A

From the Traffic surveys and analysis, it was concluded that the project road requires four lane with paved shoulders (PCUs are more than 10000), hence the Project is transferred from Ministry of Road Transport & Highways to National highways authority of India vide Gazette notification S.O. 1096 (E) dated 23.11.2016. The Tripartite agreement for the Consultancy services was signed on 04 May 2017 and the project preparation activities commenced subsequently.

As per the Tripartite agreement Project road has been divided into three packages and Package wise Details given in below table

SI.	Package		Existing km		Design C	Design	
no		Project Section	From	То	From	То	Length in km
1	Tool	Bellary to Byrapura (include Bellary Bypass)	253.600	309.150	253+600	308+550	54.95
2	İ	Byrapura to Challakere	309.150	359.120	308+550	358+500	49.95
3	ill III	Challakere to Hiriyur (Including Challakere and Hiriyur Bypass)	359.120	411.560	358+500	414+205	55.705

Current report depicts Draft feasibility report of Second package i.e. to Byrapura to Challakere.

Description of project

The State of Karnataka is located in southwest part of India. Karnataka is surrounded by Maharashtra, Goa, Andhra Pradesh, Kerala and Tamil Nadu. The NH 150A starts at Jevargi and ends at Chamarajanagar in Karnataka. The entire stretch of NH 150A passes through Gulbarga, Raichur, Bellary, Ananthpur, Chitradurga, Tumkur, Mandya, Mysore and Chamarajanagar districts and passing through important cities/towns like Jevargi, Shorapur, Lingsugur, Sindhanur, Siruguppa, Ballari, Hanagal, Challakere, Hiriyur, Huliyar, Turuvekere, Nagamangala, Pandavapura, Shrirangapattana, Mysore, Nanjanagudu and Chamarajanagar. The Total length of NH 150A is 618.62kms. The Project Road Starts at km 309.150 near Byrapura and ends at km 359.120 near Challakere. The Total Length of the project stretch is 49.95 kms. Figure 0-1 Refers to the location of the Project stretch NH 150A.

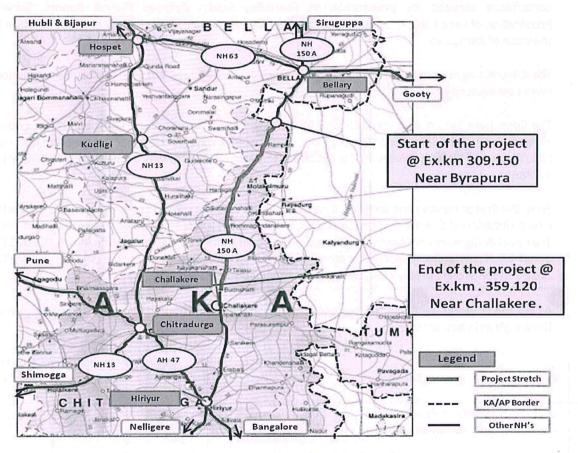


Figure 0-1: Location of Project Stretch

The project road alignment generally runs in plain terrain and passes through settlements like Byrapura, Hanagal, Mysarahatti, Rayapura, Bommagondaakere, Hirehalli Thalaku and Budnahatti

The existing alignment comprises of many sharp horizontal curves which require geometric corrections. few among these are observed to have insufficient sight distance.

The existing road has two lane carriageways with 1 m to 1.5m paved shoulder. The condition of existing pavement varies from Good to fair.

There are 98 structures present in 50.550 km length of stretch. These include 26 minor bridges, and 72 culverts.

Right of Way (RoW) available varies from 15m to 30 m in rural stretches and 15 m to 20 m in urban and Semi-urban stretches.

The traffic on this stretch of NH 150A is of mixed type, with of passenger traffic, up to 55% and that of freight traffic up to 45%. Daily traffic ranges from 12264 PCUs to 14159 PCUs, equally distributed in both direction.

The predominant land use is agricultural (73.97 % on LHS and 73.37 % on RHS) followed by Forest (16.22% on LHS and 15.02% on RHS), built up (7.61% on LHS and 7.01% on RHS) and barren land (2.20% on LHS and 4.60% on RHS)

There are 22 numbers of minor intersections & 4 numbers of major intersections along the project corridor with various categories of roads.

Large number of utility lines viz. electric / telephone lines, gas pipe lines & OFCs run parallel and across the route, which needs to be relocated for facilitating the widening.

0.2 Traffic Survey and Analysis

Traffic surveys and analyses were carried out in two phases for addressing various objectives and issues pertaining to widening of the project stretch. The surveys conducted include seven day volume counts, intersection, pedestrian/ cattle crossing, axle load and Origin Destination survey. The study aims at understanding existing traffic and travel characteristics on the project corridor and forecasting for project horizon year considering various constituent streams and for various scenarios. The results of analysis would form inputs for designing the pavement, developing capacity augmentation proposals, carrying out financial analysis, decisions regarding grade separators, pedestrian facilities, planning the tolling strategy, designing the toll plaza, wayside amenities along with design of intersections on the widened project road.

The volume count surveys were conducted at three locations, i.e., in july month of 2015 & May month of 2016. The annual average daily traffic at these locations is shown in table below.

Table 0-1 Annual Average Daily Traffic along project corridor

Mada of Vahiala	km 311.400	km 327.750	km 350.300
Mode of Vehicle	Hanagal	B.G Kere	Thalaku
Car / Jeep / Van (Private)	1064	1273	1281
Car / Jeep / Van (Taxi)	194	294	330
Local/Shared Car	13	19	105
Mini Bus	28	40	38
School Bus	2	4	2
Govt. Bus	280	312	345
Pvt. Bus	243	274	273
LMV	199	311	349
LCV (4 Wheels)	142	72	67
LCV (6 Wheels)	188	135	194
2 Axle	286	368	347
3 Axle	684	670	720
MAV (4 - 6 Axles)	1158	1144	1212
MAV (7++ Axles)	0	0	0

Made of Vehicle	km 311.400	km 327.750	km 350.300
Mode of Vehicle	Hanagal	B.G Kere	Thalaku
2 Wheeler	1258	1395	1601
3 wheeler/Auto	247	199	161
Tractor	10	22	15
Tractor with trailer	81	76	115
Cycle	10	27	6
Cycle rickshaw	0	0	0
Animal drawn	2	10	1
Toll exempted Vehicles	33	24	41
Motorised	6110	6632	7196
Non motorised	12	37	7
Tollable Vehicles	4481	4916	5263
Non-Tollable Vehicles	1641	1753	1940
Total Vehicles	6122	6669	7203
Total PCU	13023	13687	14590

The investment priorities are governed by traffic demand, assessed benefits and cost of project. Demand plays the important role that governs which type of facility / infrastructure to be created. This in turn determines likely benefits and costs to develop the same. A highway project of this nature calls for significant investment. Prediction of traffic demand becomes an important task and has to be carried out near accurately. For the design of pavement and to plan for the future maintenance programme as well as capacity augmentation and for financial evaluation, it is necessary to have realistic estimate of the size of traffic in horizon year.

Traffic forecasting is done in two ways, one with CAGR of 5 % as proposed in the ToR and the other with growth rates established by using elasticity method. This forecasting is made by determining the past trend of traffic flow along the corridor and by use of economic models developed to corelate past vehicle registration data and economic indices such as per capita income (PCI), net state domestic product (NSDP) and gross domestic product (GDP). By using the elasticity values obtained from the econometric models and the likely rate of growth of indices, population and regional influences, the mode wise growth rates are established.

Classified direction wise turning movement survey has been conducted at 4 major junctions shown in Table below along the project stretch. Type of existing intersection and structures proposed based on the turning movement survey are also shown in Table below.

Table 0-2 Turning movement survey locations

S No.	Chainage	Location	Total Volume PCU	Peak Hour Volume PCU	Peak Hour
1	314.220	Near Hanagal	10256	1233	18.00-19.00
2	314.770	Hanagal Cross	7936	884	20.00-21.00
3	317.760	Rayapura Cross	6883	10236	18.00-19.00
4	347.170	Garani Cross	7082	11414	18.00-19.00

0.3 Axle load survey

Axle load survey has been conducted at 1locations using portable electronic weigh pads in order to plot actual loading spectrum of commercial vehicles, which create potential damage to pavement.

This damage to pavement normally qualified by a factor termed as vehicle Damage Factor (VDF). The VDF for project stretch is worked out using equivalency factors and damaging power of different axle, IRC: 37-2012. The VDF values adopted for MSA calculation are shown in Table below.

The Axle load surveys were conducted at one location, i.e., in may month of 2016. The annual average daily traffic at these locations is shown in table below.

Table 0-3 VDF values, observed and recommended

Location	Existing NH 150A km 350.300				
Mode	Byrapura – Hiriyur	Hiriyur - Byrapura	Recommended		
LCV	1.02	0.41	1.02		
2-axle	2.96	1.69	2.96		
3-axle	3.16	3.70	3.70		
MAV	5.85	3.25	5.85		
Bus	1.45	1.25	1.45		

The cumulative million standard axles at count locations have been calculated for various horizon years and shown in Table below. These values are used in new pavement design.

0.4 Intersections

There are about 26 intersections along the project stretch includes 3 major Intersections and 22 minor Intersections. The minor & major intersections with state highways and district roads are given in below table

Table 0-4 List of Minor Junctions

SI.	Existing	Type of	Le	ads To	Type of
No	km	Junction	LHS	RHS	Intersection
1	311.060	3 legged	Katanaikanahalli		Tuling
2	311.970	3 legged	Katanaikanahalli		T
3	313.207	3 legged	Pujarihatti	All year Substitute of	make Tolling
4	314.250	3 legged		Bommalinganahalli	Υ
5	314.660	3 legged		Village road	T
6	316.445	3 legged	I Complete Ships in	Iyanahalli	T
7	317.150	3 legged	Rayapura		T
8	320.530	4 legged	Nerlahalli	Tumkurlahalli	Staggered, +
9	324.890	3 legged	Marammanahalli		nas segeTess— e n
10	331.010	4 legged	Mogalahalli	Surammanahalli	Staggered, X
11	331.755	3 Legged	Village Road		Υ
12	331.940	3 legged		Bommagondanakere	Т
13	338.305	3 legged	Village road	The state of the s	T
14	339.030	4 legged	Village road	Hirehalli	Staggered, +
15	339.850	4 legged	Chikkahalli	Hirehalli	Staggered, +
16	345.160	4 legged	Byadareddyhalli	Kereyagalahalli	Staggered, X
17	345.850	3 legged	Mannekote		Т

SI.	Existing	Type of	Leads To		Type of
No	km	Junction	LHS	RHS	Intersection
18	346.490	. 3 legged	Village road	the bestern of the state of	T
19	348.260	3 legged	Thalaku	ipkrhium ensyr gilden	un berek ettel en
20	351.320	4 legged	Village road	Devarhalli	+
21	354.145	3 legged	tales and a military	Varavoo	Т
22	358.100	3 legged		Labanehatti	Y

Table 0-5 List of Major Junctions

SI.	Existing	Type of	Leads	То	
No			LHS	RHS	Remarks
1	314.860	4 Legged	Mulakalamuru road	Village road	
2	317.870	4 Legged	Rayapura	Yerenahalli	SH-65
3	347.170	3 Legged		Nayakanhatti	SH-65

0.5 Engineering Surveys and Investigations

The general engineering studies conducted along the project stretch include topographic survey, Pavement composition, pavement condition surveys and pavement structural strength.

Topographic Survey

The Topographic Survey for improvement of project road was carried out by GPS, electronic Total Station and Auto level equipments. The detailed survey methodology and specifications followed are as described below.

Initially control points and traverse stations were established using GPS and Total Station equipments. Then using Auto level equipment, Double Tertiary (DT) levelling was carried out to fix temporary benchmarks with respect to GTS Benchmarks available in the area to establish the vertical control to all TBMs and traverse stations. The detailed survey was then carried out and the data was processed for engineering design.

The following specifications were adopted for carrying out the topographic surveys. Traverse points positioning was with accuracy of 1:10,000. All traverse points were marked on ground by punching nails in to surface. TBMs are fixed on BM reference pillars fixed 250 m apart, at top of the kilometre stones, parapets of culverts and on plinth top of the buildings. Double tertiary methods have been used in levelling, taking the sum and average of three wire readings to determine reduced levels.

Cross-sections of roads are taken at an interval of 50 m. Levels were taken at the centre and edges of carriageway, edge of shoulders and levels up to property lines on both sides. Details of drain widths and depths also collected. For all the existing culverts, top levels / road levels and bed levels / sill levels were collected.

Road Inventory

The existing pavement for the entire stretch is of bituminous surface except at major bridge locations. The pavement width is predominantly two lane carriageway with 1.0 m Paved shoulders on either side and 4 lane divided carriageway exists at some built up stretches of the highway and is varies from fair to poor condition.

Pavement Condition

The survey on general pavement condition was primarily a visual exercise undertaken by means of slow drive-over survey, and supplemented with measurements wherever necessary. Visual assessment was carried out at suitable intervals at 200 m and wherever necessary, depending on variations in pavement conditions.

The existing pavement condition based on visual observation varies from good to poor. It has been observed that in most of the stretches heaving / settlements / distress were observed. This may be due to weak subgrade, improper compaction or movement of heavy loaded trucks. Summary of pavement condition is given in Table below.

Table 0-6 Summary of Pavement Condition

SI. No.	Summary	Length, (km)	%
1	Good	28.85	57.76
2	Fair	21.10	42.24
Total		49.95	100.00

Pavement Structural Strength

Benkelman Beam deflection studies were carried out for evaluating the residual strength of the existing pavement and assessing the strengthening requirements for the existing pavement.

BBD tests have been conducted for every 3 km interval. The pavement deflection has been observed for homogeneous sections based on pavement condition survey.

The summary of Characteristic deflection is given in Table below

Table 0-7 Summary of Characteristic Deflection

SI.	Existing Km	NH-150A	Characteristic
No	From	То	Deflection (mm)
1	308.500	308.725	1.66
2	311.400	311.625	0.99
3	316.750	316.975	1.13
4	320.750	320.975	2.47
5	322.750	322.975	1.26
6	326.750	326.975	2.06
7	329.750	329.975	1.76
8	332.750	332.975	1.60
9	336.750	336.975	2.22
10	339.700	339.925	0.83
11	342.750	342.975	1.80
12	345.770	345.995	1.58
13	351.000	351.225	1.94
14	353.800	354.025	1.02

SI.	Existing Kn	n NH-150A	Characteristic
No	From	То	Deflection (mm)
15	358.200	358.425	2.12

Subgrade Investigation

The laboratory investigations of sub-grade indicate that the existing subgrade varies from location to location along the road. The 4 days soaked CBR values for 65 blows of existing sub-grade varies from 9% to 11%.

Material Investigation

Potential sources of soil for construction of embankment and subgrade (for reconstruction / new carriageway) were identified on either side of project stretch.

Borrow area quarries located in 6 locations with average CBR of 10% which is found suitable for construction.

Aggregate quarries were identified in 5 locations out of these 4 locations found suitable for construction.

Hydrological investigations

Hydrological investigations have been carried out for the entire project road. It has been verified that majority of the cross drainage structures are hydrologically adequate to carry the discharges of the river / streams. It has been ascertained from local enquiry and from the National Highways.

0.6 PRELIMINARY DESIGN

Geometric design

The existing alignment largely runs is in plain terrain and the design speed of 80 to 100 kmph has been adopted as per IRC: SP: 84- 2014. Geometric design viz. horizontal and vertical curves are designed as per IRC 38-1988 & IRC: SP: 23 - 1983.

Alignment proposal

After carrying out field investigations and reconnaissance survey of existing alignment, the consultants have been arrived at alignment proposals.

Widening proposals have been prepared based on availability of additional land / existing RoW, horizontal geometrics, study of existing bridges and considering road safety parameter measures. The lengths of adopted type of widening details (as per proposed chainage) are given in Table below.

Table 0-8 Length of adopted type of widening scheme

SI. No	Type of widening	Design Length km
1	Concentric	8.070
2	Eccentric	24.800
3	Realignments	16.270
4	Toll plaza	0.810
	Total Length, Km	49.950

Proposed Lane widths

The width of basic traffic lane is taken to be 3.5 m. For proposed 4-lane the carriageway width will be 14m width with paved shoulders on both sides. In divided cross-section, the median will be of 5.0 m width in rural areas and of 2.5 m width in urban areas including 0.5 m shy off on both sides. The proposed carriageway details are presented in table below.

Table 0-9 Proposed Lane widths

C N-	Design (Chainage	Longth m	Lane Width	Remarks
S. No	From	То	Length, m	Lane Width	Kelliaiks
1	308+550	312+460	3910	4 Lane	
2	312+460	313+000	540	4 Lane with service road	Pujarihatti
3	313+000	316+950	3950	4 Lane	
4	316+950	317+250	300	4 Lane with service road	Mysavahatti
5	317+250	331+320	14070	4 Lane	
6	331+320	332+000	680	4 Lane with service road	Bommagondanakere
7	332+000	358+500	26500	4 Lane	

Typical Cross Sections

Based on the traffic considerations, geometric standards and existing site conditions, the following typical cross sectional elements are framed for project road.

Table 0-10 Details of Proposed Cross Section

Element	Width (m)	Total Width (m)
4-lane Divided Carriage way Cross Section in	n Rural (C/S Type 1,1A,1E	3,& 1D)
Main Carriageway	2 X 7.00	14
Median	1 x 4.00	4
Kerb Shyness	2 x 0.5	1
Paved shoulder	2 X 1.50	3
Earthen Shoulders	2 X 2.00	4
Utility corridor	2 x2.00	4
Drain, Future widening etc	2x15.0	30
Total	nng. 157 bre bendings	60
4-lane Divided Carriage way Cross Section	in Urban (C/S Type 2, 2A	& 2B)
Main Carriageway	2 X 7.00	14
Median	1 X 1.50	1.5
Kerb Shyness	2 x 0.5	1
Paved shoulder	2 X 2.0	4
Earthen Shoulders	2 X 2.00	4
Earthen Shoulders for service roads	2 X1.50	3
Service Road	2 X 7.5	15
Drain cum Foot Path	2 X 1.5	3
Utility Corridor/Footpath	2 x 2.00	4
Space left for future widening / Drain	2 X 5.25	10.5
Total		60

Element	Width (m)	Total Width (m)
VUP approach Cross Section in ru	ral (C/S Type 3)	A margarette Ties
Main Carriageway	2 x 10.5	21
Median	1 x 4.00	4
Median Shy off	2 x 0.5	1
Paved shoulder	2 X 2.0	4
Crash barrier with shyness	2 x 1.0	2
Slip road on both sides	2 x 7.0	14
Earthen shoulders	2 x 1.5	3
Divider between slip Road and VUP Approach	2 x 0.5	1
Utility Corridor	2 x 2.0	4
Drain & Slope	2 x 3.0	6
Total		60
VUP/Flyover approach Cross Section in	Urban (C/S Type 3A)	
Main Carriageway	2 x 10.5	21
Median	1 X 1.50	1.5
Median Shy off	2 x 0.5	1
Paved shoulder	2 X 2.0	4
Crash barrier with shyness	2 x 1.0	2
Divider between slip Road and Fly Over Approach	2 x 4.250	8.5
Service road on both sides	2 x 7.5	15
Utility corridor	2 x 2	4
Footpath cum drain	2 x 1.5	3
Total	er ny Franti hanai 40 n. il	60
VOP Approach Cross Section in Ur	ban (C/S Type 4)	rigetant (ECC)
Main Carriageway	2 x 14.00	28
Median	1 x 4.00	4
Median Shy off	2 x 0.5	1
Paved shoulder	2 X 1.5	3
Footpath cum Drain	2 x 1.5	3
Crash Barrier	2 x 0.5	1
Divider between slip Road and VOP Approach	2 x 0.5	1
Divider Shy Off	2 x 0.5	1
Slip road on both sides	2 x 5.5	11
Kerb Shyness	2 x 0.5	1
Footpath cum Drain	2 x 1	2
Utility Corridor	2 x 2.0	4
Total		60
4-lane Divided Carriage way Cross Section	in Hilly Area (C/S Type	: 5)
Main Carriageway	2 X 7.00	14
Median	1 x 4.00	4
Kerb Shyness	2 x 0.5	1
Paved shoulder	2 X 1.50	3
Earthen Shoulders	2 X 2.00	4

Element	Width (m)	Total Width (m)
Utility corridor	2 x2.00	4
Drain, Breast Wall, Catch water Drain and Future widening etc	2x15.0	30
Total		60

Pavement Design

New Flexible pavement has been proposed for additional 2-lane & flexible overlay over existing 2-lane road. Rigid pavement is proposed only at Major realignments and toll plaza location.

New Flexible/Rigid Pavement Design

The pavement design basically aims at determining the total thickness of the pavement structure as well as thickness of individual structural components. The following assumptions are considered for the preliminary pavement design. The basic assumptions considered while designing are as follows.

As per IRC: SP: 84-2014 (for 4-Lane)

As per IRC: SP: 84-2014 flexible pavements shall be designed for a minimum design period of 15 years or operation period whichever is more. Stage construction will be permitted subject to the thicknesses of sub-base & base coarses are designed for 15 years & bituminous surface for a minimum of 10 years. Strengthening for the future traffic can be carried out by means of Bituminous Overlay. Rigid pavement shall be designed for a minimum design period of 30 years; stage construction shall not be permitted.

Flexible Pavement

The project road has been divided into three homogeneous sections, design for which are furnished below.

Table 0-11 Cumulative MSA at sections

OF COMPANY TO SERVICE OF THE SERVICE	Sect	Section-I		Section-II		Section-III	
Description	LHS	RHS	LHS	RHS	LHS	RHS	
Design life of Base and Sub Base (in years) - For Stage Const.	15	15	15	15	15	15	
Design life of BT layer (in years) - For Stage Const.		10	10	10	10	10	
MSA for Base and Sub Base - For Stage Const.		50.98	45.01	48.9	47.18	51.9	
MSA for BT layer - For Stage Const.		30.29	26.77	29.07	28.05	30.84	
Design CBR %	9	9	9	9	9	9	

Table 0-12 Flexible pavement composition (in mm) recommended for Main Carriageway

Description	Pavement Thickness, mm				
Description	Byrapura-Challakere	Challakere to Byrapura			
Bituminous Concrete (BC), mm	40	40			
Dense Bituminous Macadam , mm	95	85			
Wet Mix Macadam , mm	250	250			
Granular Sub Base, mm	200	200			

0-11

是125. 13. 14. 14. 14. 15. 15. 15. 15. 15. 15. 15. 15. 15. 15	Pavement Thickness, mm				
Description	Byrapura-Challakere	Challakere to Byrapura			
Subgrade of CBR 9% material, mm	500	500			
Total	1085	1075			

The pavement composition of paved shoulders has been kept with the same specifications as those of the main carriageway.

Table 0-13: Flexible Pavement Compositions for Service road

THE STREET	Servic	e/Slip Roa	d extilland a man
Composition	MSA	Design CBR (%)	Thickness, mm
ВС	era	Tare menara	40
DBM	11 2010		50
WMM	10	0	250
GSB	10	9	200
Subgrade	uls her f		500
Total	r. 5.375		1040

Table 0-14 Overlay Thickness (in mm) recommended

s.	NH-150A Existing Km		Design Chainage		Average Characteristic	DACA.	вм	Overlay	Des Thick	
No	From	То	From	То	Deflection	MSA	(mm)	(mm)	BC (mm)	DBM (mm)
1	309.150	316.950	308+550	316+850	1.02	37	89.84	63	40	50
2	316.950	326.950	316+850	326+930	1.75	35.53	176.44	124	40	85
3	326.950	346.950	326+930	346+600	1.44	35.53	149.48	105	40	65
4	350.200	359.100	349+700	358+500	1.63	38.16	169.05	118	40	80

Rigid Pavement

The Rigid pavement is proposed at Built up locations and toll plaza location. The design as per the IRC: 58 – 2015 leads to the crust thickness of pavement as given in following table.

Table 0-15 Rigid Pavement Compositions (As per IRC: 58-2015)

For CBR = 9%

Pavement Composition	Thickness in mm
Design Life (Yrs)	30
Design CBR (%)	10
PQC (mm)	280
DLC (mm)	150
GSB (mm)	150
Sub-grade with material having effective CBR of 9%	500
Total	1080

Truck lay-byes

Truck lay-byes are proposed at following locations and new Flexible pavements is proposed at these locations.

SI. No.	Existing Km	Design Chainage	Side	Name / Location
1	316.600	316+500	Both	Near Mysarahatti
2	354.370	354+800	Both	Near Chikkammanahalli

Rest Area

The location of the Rest area is given below

S. No	Existing km	kisting km Design Chainage Side		Location	Area	
1	332.725	332+600	LHS Near Bommagondanakere		3 Ha	
2	332.980	332+850	RHS Near Bommagondanakere		3 Ha	

Bus Bay with Bus Shelters

There are 12 nos Bus Bay with Bus Shelters and 22 bus shelters are proposed along the project stretch.

Toll Plaza

Rigid Pavement is proposed for the toll plaza location, as it has longer life and can resist the wear and tear caused by the braking forces exerted by heavy vehicles.

One toll plaza is proposed with additional right-of-way, service lanes, toll booths, lighting, weigh-in-motion Weigh Bridge, automatic, semi automatic and manual toll booths, separate lanes for wide bodied vehicles etc. The detail of the proposed toll plaza is given below.

Table 0-16 Proposed Toll Plaza Location

	Toll Plaza Location			No of To	oll Lanes*
Sl. No.	Existing Chainage	Design Chainage	Location	LHS	RHS
1	341.560	341+300	Near Chikkamanahalli	7+1	7+1

^{*}As per DO No. NHAI/Chairman/Misc./2016 dated 26.12.2016

Proposal for Structures

There are 30 no's. Minor bridges, 4 no.'s Flyovers, 1 no. VOP, 6 no's LVUP's along with many other cross drainage works are proposed along the project stretch. Nos. of each type of structure along the project stretch is given in below Table. These structures are proposed for widening, rehabilitation or construction of new structure.

Table 0-17: Proposed Structures on Project Stretch

SI. No.	Type of structure	No's.
1	Minor bridges	30
2	Flyover	04
3 VOP		01
4 LVUP's		06
5 Culverts		78
6 Cross Road Culverts		31

0.7 Cost Estimation

The cost estimation for the project is extremely important as the viability and implementation of a project depends on the project cost. Therefore, cost estimates have been carried out with due care. Estimation of preliminary cost, a primary pre-requisite for economic and financial evaluation, has been carried out for widening the existing NH stretches to 4-lane carriageway with paved shoulders on both sides including Reconstruction of the existing pavement, strengthening / widening of existing bridge structures, construction of new bridges, rehabilitation and reconstruction / widening of cross drainage structures, longitudinal drains, junction improvements, road furniture, bus bays, truck bays, way side amenities, toll plazas, etc. and is presented in below Table.

0. EXECUTIVE SUMMARY

0.1 Introduction

The Highway starting from Jeevargi connecting Maski, Siruguppa, Bellary, Challakere, Hiriyur, Turuvekere, Shrirangapattana, Mysore, and Chamarajanagar, which previously comprised of SH was recently upgraded and declared as NH-150A by Ministry of Road Transport & Highways (MoRT&H), Government of India (GOI).

The Ministry of Road Transport & Highways (MoRT&H), Government of India (GOI), (Public Works, Ports And Inland Water Transport Department), Office of The Executive Engineer, National Highways, Chitradurga division has appointed M/s. Feedback Infra Pvt. Ltd. for providing the consultancy services for preparation of Feasibility Study, Detailed Project Report, Survey, Preparation of Land Plan for widening to Two lane Paved shoulder from Bellary to Hiriyur section in the state of Karnataka.

The Contract Agreement for the assignment was signed on 7th May 2015; the Consultancy services were commenced with effect from 7th May 2015.

The Consultant has studied the feasibility of the project and submitted the Detailed Project Report along with EPC Schedules Executive Engineer, National Highways, Chitradurga division vide letter no FIPL/Highways/DPR/NH-PWD/CTR/2016-17/485 and FIPL/Highways/DPR/NH-PWD/CTR/2016-17/486A

From the Traffic surveys and analysis, it was concluded that the project road requires four lane with paved shoulders (PCUs are more than 10000), hence the Project is transferred from Ministry of Road Transport & Highways to National highways authority of India vide Gazette notification S.O. 1096 (E) dated 23.11.2016. The Tripartite agreement for the Consultancy services was signed on 04 May 2017 and the project preparation activities commenced subsequently.

As per the Tripartite agreement Project road has been divided into three packages and Package wise Details given in below table

SI.			Existing km		Design Chainage		Design
no Package	Project Section	From	То	From	То	Length in km	
1	ı	Bellary to Byrapura (Include Bellary Bypass)	253.600	309.150	253+600	308+550	54.95
2	П	Byrapura to Challakere	309.150	359.120	308+550	358+500	49.95
3	Ш	Challakere to Hiriyur (Including Challakere and Hiriyur Bypass)	359.120	411.560	358+500	414+205	55.705

Current report depicts Draft feasibility report of Package-III i.e. to Challakere to Hiryur (Including Challakere and Hiriyur Bypass).

Table 0-18 Summary of Cost Estimate for Byrapura to Hiriyur section of NH-150A

ABSTRACT OF COST - Byrapura - Challakere Section of NH-150A (Ds.Ch: 308+550 to Ds.Ch: 358+500)

BILL NO.	BILL NAME		Total Amount in Rs	Total Amount in Crores	Total Amount in %
	CIVIL CONSTRUCTION COST	named in America			
1	Site Clearance and Dismantling	2,14,04,901	2.14	0.35%	
2	Earth Work		69,18,31,848	69.18	11.40%
3	Granular Sub-Base and Base Courses		1,10,72,27,125	110.72	18.25%
4	Bituminious Works		-		
4A	Flexible Pavement		1,17,04,29,993	117.04	19.29%
5	CULVERTS			100	
5A	Box Culvert	and the same of	17,40,13,010	17.40	2.87%
5B	Pipe Culvert	TO SECURE SEC.	6,45,66,468	6.46	1.06%
6	BRIDGES	tent the test		All was 1 - Mr.	er aller inc.
6A	Repair & Rehabilitation of Structures		7,98,856	0.08	0.01%
6C	Minor Bridges		90,69,32,746	90.69	14.94%
6E	PUP/LVUP	44,42,32,332	44.42	7.32%	
6F	CUP	9,79,14,064	9.79	1.61%	
6H	Flyover and Overpass		62,08,34,286	62.08	10.23%
7	Drainage, Protective Works & Other Services	17,52,04,392	17.52	2.89%	
8	Junctions	8,52,89,890	8.53	1.41%	
9	Traffic Signs, Road Marking and Other Appurtenances	19,59,91,350	19.60	3.23%	
10	Miscellaneous		20,20,87,150	20.21	3.33%
11	Toll Plaza Construction @ Ch.341+300	MARKET OF THE	10,98,07,651	10.98	1.81%
Α	Total Civil Cost	(YR: 2017-2018) =	6,06,85,66,062	606.86	
	Civil Cost per Km (Length of Project	Highway-50.05 km)	12.13		and the support
11	Centage (As per ministry circular dated 10th Aug 201	6, for HAM project	91,02,84,909	91.03	DE BELL
III	TOTAL PRO	DJECT COST (IV+VIII)	6,97,88,50,972	697.89	FELTA FE
	Total	Project Cost per Km	13.94		
	PRE CONSTRUCTION ACTIVITY			1	
	LA, R&R and Social	The same of the	1,39,49,02,850	139.49	
IV	Environment Cost and Utility shifting and other preconstrucion activities	da Luma i	1,15,55,710	1.16	de recen
	Shifting of Electrical Poles/Lines (1% of Civil Cost) =		6,06,85,661	6.07	and the same of the
	Shifting of Water Supply Pipe Lines (1	6,06,85,661	6.07		
	Total cost of preconstruction activities I	1,52,78,29,881	152.78		
V	Total Capital Cost (C+D) =		8,50,66,80,853	850.67	1 1.
	Total Capital Cost	Per Km in Crores =	17.00		

CONCLUSIONS AND RECOMENDATION

ECONOMIC ANALYSIS

The economic internal rate obtained was return of 25.7% for development of the project road with flexible pavement, which is more than the presented 12% of discounted rate and larger positive value of NPV of net benefits indicate the firm viability of project for the proposed improvement of four laning with paved shoulders and hence recommended for implementation.

FINANCIAL ANALYSIS

It is concluded that the project doesn't provide sufficient returns to equity on commercial BOT (Toll) format with 40 % grant for concession period of 10 years & 15 years. However, if the concession period is increased to 20 years, the project provides sufficient returns to equity at 7.5% Grant. For a concession period of 25 Years, the project doesn't require any grant and provides 16.38% return on equity.

However, the consultants recommend the project on HAM with concession period of 15 years and on EPC mode, as per Policy Matter: Technical ((161/2014) Lt No.11041/218/2007-Admn dated 24.07.2014.

National Highways Authority of India

Project Director

Description of project

The State of Karnataka is located in southwest part of India. Karnataka is surrounded by Maharashtra, Goa, Andhra Pradesh, Kerala and Tamil Nadu. The NH 150A starts at Jevargi and ends at Chamarajanagar in Karnataka. The entire stretch of NH 150A passes through Gulbarga, Raichur, Bellary, Ananthpur, Chitradurga, Tumkur, Mandya, Mysore and Chamarajanagar districts and passing through important cities/towns like Jevargi, Shorapur, Lingsugur, Sindhanur, Siruguppa, Ballari, Hanagal, Challakere, Hiriyur, Huliyar, Turuvekere, Nagamangala, Pandavapura, Shrirangapattana, Mysore, Nanjanagudu and Chamarajanagar. The Road was declared as National Highways No. 150A. The Total length of NH 150A is 618.62 Kms. The Project Road Starts at Km 359.120 of NH-150A near Challakere and ends at Km 411.560 of NH-150A Hiriyur. The total existing length of the project stretch is 52.44 km and mainly passes through Challakere, Sanikere, Hirehal and Hiriyur. Figure 0-1 Refers to the location of the Project stretch of NH 150A.

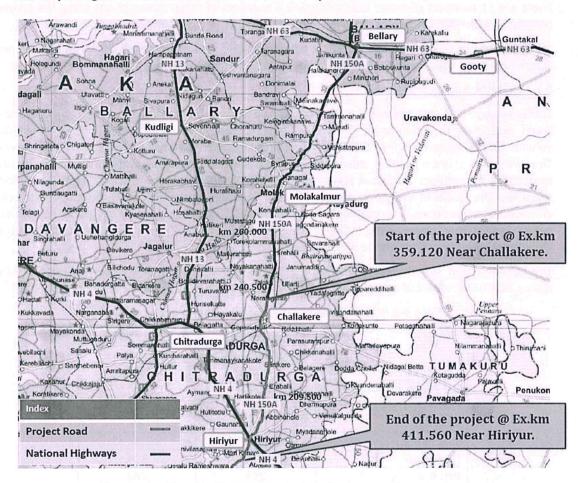


Figure 0-1: Location of Project Stretch

The project road alignment generally runs in plain terrain and passes through settlements like Challakere, Samikere, Hirehal, Hiriyur.

The existing alignment comprises of many sharp horizontal curves which require geometric corrections. Few among these are observed to have insufficient sight distance.

The existing road has two lane carriageways with 0.5m to 1.5m earthen shoulder. The condition of existing pavement mainly varies from Good to fair.

There are 81 structures present in 52.44 km length of stretch. These include 1 major bridge, 24 minor bridges, 1 VOP, 1 RUB and 54 culverts.

Right of Way (RoW) varies from 10 m to 30 m in the project stretch.

The traffic on this stretch of NH 150A is of mixed type, with of passenger traffic is varying from 51.7 % to 56.4 % and that of freight traffic range from 43.6 % to 47.9 %. Share of slow moving vehicles range from 0.0 to 0.4 %.

The predominant land use is agricultural (77.30 % on LHS and 73.50 % on RHS) followed by built up (22.20 % on LHS and 25.50 % on RHS) and Industrial (0.50 % on LHS and 1.00 % on RHS)

There are 51 numbers of minor intersections & 4 numbers of major intersections along the project corridor with various categories of roads.

Large number of utility lines viz. electric / telephone lines, gas pipe lines & OFCs run parallel and across the route, which needs to be relocated for facilitating the widening.

0.2 Traffic Survey and Analysis

Traffic surveys and analyses were carried out in two phases for addressing various objectives and issues pertaining to widening of the project stretch. The surveys conducted include seven day volume counts, intersection, pedestrian/ cattle crossing, axle load and Origin Destination survey. The study aims at understanding existing traffic and travel characteristics on the project corridor and forecasting for project horizon year considering various constituent streams and for various scenarios. The results of analysis would form inputs for designing the pavement, developing capacity augmentation proposals, carrying out financial analysis, decisions regarding grade separators, pedestrian facilities, planning the tolling strategy, designing the toll plaza, wayside amenities along with design of intersections on the widened project road.

The volume count surveys were conducted at two locations, i.e., in July month of 2015 & May month of 2016. The annual average daily traffic at these locations is shown in table below.

Table 0-1 Annual Average Daily Traffic along project corridor

NA - 1 - CV-11-1	Km 376.220	Km 391.285
Mode of Vehicle	Mummadisagara B	Harthikote
Car / Jeep / Van (Private)	1082	1069
Car / Jeep / Van (Taxi)	378	381
Local/Shared Car	54	38
Mini Bus	43	38
School Bus	2	1
Govt. Bus	304	314
Pvt. Bus	253	262
LMV	256	420
LCV (4 Wheels)	96	101
LCV (6 Wheels)	178	167
2 Axle	416	472
3 Axle	682	727
MAV (4 - 6 Axles)	1063	1115

Made of Valid	Km 376.220	Km 391.285
Mode of Vehicle	Mummadisagara B	Harthikote
MAV (7++ Axles)	0	0
2 Wheeler	1360	1158
3 wheeler/Auto	115	91
Tractor	7	20
Tractor with trailer	35	48
Cycle	0	22
Cycle rickshaw	0	0
Animal drawn	2	3
Toll exempted Vehicles	21	7
Motorised	6345	6429
Non motorised	2	25
Tollable Vehicles	4807	5104
Non-Tollable Vehicles	1540	1349
Total Vehicles	6347	6454
Total PCU	13003	13668

The investment priorities are governed by traffic demand, assessed benefits and cost of project. Demand plays the important role that governs which type of facility / infrastructure to be created. This in turn determines likely benefits and costs to develop the same. A highway project of this nature calls for significant investment. Prediction of traffic demand becomes an important task and has to be carried out near accurately. For the design of pavement and to plan for the future maintenance programme as well as capacity augmentation and for financial evaluation, it is necessary to have realistic estimate of the size of traffic in horizon year.

Traffic forecasting is done in two ways, one with CAGR of 5 % as proposed in the ToR and the other with growth rates established by using elasticity method. This forecasting is made by determining the past trend of traffic flow along the corridor and by use of economic models developed to corelate past vehicle registration data and economic indices such as per capita income (PCI), net state domestic product (NSDP) and gross domestic product (GDP). By using the elasticity values obtained from the econometric models and the likely rate of growth of indices, population and regional influences, the mode wise growth rates are established.

Classified direction wise turning movement survey has been conducted at 3 major junctions shown in Table below along the project stretch. Type of existing intersection and structures proposed based on the turning movement survey are also shown in Table below.

Table 0-2 Turning movement survey locations

Location	Chainage (NH 150A)	Survey Location	Type of Junction	Legs leading to	Date
TMC1	km 361.650	Near Challakere	3 Leg	Kalyanadurgam	17-07-2015
TMC 2	km 363.485	Challakere Junction	4 Leg	Pavagada, Chitradurga	18-07-2015
TMC 3	km 404.250	Hiriyur T.B Circle	4 Leg	Chitradurga, Bangalore	20-07-2015

0.3 Axle load survey

Axle load survey has been conducted at 1locations using portable electronic weigh pads in order to plot actual loading spectrum of commercial vehicles, which create potential damage to pavement.

This damage to pavement normally qualified by a factor termed as vehicle Damage Factor (VDF). The VDF for project stretch is worked out using equivalency factors and damaging power of different axle, IRC: 37-2012. The VDF values adopted for MSA calculation are shown in Table below.

The Axle load surveys were conducted at one location, i.e., in May month of 2016. The annual average daily traffic at these locations is shown in table below.

km 376.220 Location Mode Ballari – Hiriyur Hiriyur - Ballari Recommended LCV 1.11 0.63 1.11 3.14 2-axle 3.14 1.76 3-axle 3.73 1.73 3.73 MAV 2.81 6.16 6.16 Bus 1.30 1.29 1.30

Table 0-3 VDF values, observed and recommended

The cumulative million standard axles at count locations have been calculated for various horizon years and shown in Table below. These values are used in new pavement design.

0.4 Intersections

There are about 55 intersections along the project stretch includes 4 major Intersections and 51 minor Intersections. The minor & major intersections with state highways and district roads are given in below table

SI. No	Full state of law	Type of	Leads To		
	Existing km	Intersection	Left	Right	
1	362.100	Staggered	To Durgavara	To Sujimalleswara Nagar	
2	362.315	3 Legged	Jun Williams en Sillaës	To Sujimalleswara Nagar	
3	362.820	Staggedred	To Katappana hatti	To Chitrayyanna hatti	
4	363.075	3 Legged	To Challakere Old Town	A cook a rife or skiller a	
5	363.240	3 Legged	To Challakere Old Town		
6	363.300	3 Legged	To Challakere Old Town	- 1	
7	363.700	3 Legged	To Railway Station Road	- , - ,	
8	363.860	3 Legged	See Heart St. F.	To Gandhi Nagar	
9	363.975	3 Legged	To Challakere Town	To his city	
10	364.220	Staggered	To Challakere Town	To Gandhi Nagar	
11	364.300	3 Legged	To Challakere Town	-	
12	364.385	3 Legged	To Challakere Town		
13	364.465	3 Legged	7 - N . m. n '- '	To Gandhi Nagar	
14	364.560	3 Legged	To Challakere Town		
15	364.685	3 Legged	-	To Gandhi Nagar	
16	365.490	3 Legged	To Nagaramgere	-	

Table 0-4 List of Minor Junctions

CL N-	Type of		Leads To		
SI. No	Existing km	Intersection	Left	Right	
17	366.755	3 Legged		To Village	
18	366.605	4 Legged	To SR Layout	To Radhaswamy Nagar	
19	368.350	4 Legged	To Siddapur	To Laxmipur	
20	370.255	3 Legged	- middlestand me	To Hotteppanahalli	
21	373.125	3 Legged	To Gopanahalli	and the state of t	
22	374.465	3 Legged	de la marina editra maja social	To Ganugutta	
23	376.260	4 Legged	To Golarahatti	To Sanikare	
24	377.610	3 Legged	To Chikkanahalli	The State of the S	
25	377.705	3 Legged	To Sondekere	i and the terms of the second	
26	379.540	3 Legged		To Heggere	
27	385.375	3 Legged	•	To Maddanakunte	
28	386.550	3 Legged	To Kandikere	<u> </u>	
29	387.370	3 Legged	AND THOMBS SECTION	To Vaddikere	
30	388.810	3 Legged	To Gudunurhalli	The Part 2019 Heller	
31	389.095	3 Legged	Brand Similar Ballindan	To Kalavibagi	
32	390.280	3 Legged	To Rangenahalli	Market de la desenta mariane	
33	391.335	4 Legged	To Harthikotta	To Salavammana Halli	
34	396.430	3 Legged	To Balenahalli	a again Pagagaga Resalla (Austria	
35	397.270	3 Legged	To Balenahalli	i distribui san i sti se seg	
36	399.000	3 Legged	1 . I . S. S. S (1891	To Gannaanakamhalli	
37	399.340	3 Legged	To Hamadala Village	The Mark Street Company Control of the Control of t	
38	400.045	3 Legged	Kertija nggariya ar ara	To Chinanaikanhatti	
39	400.950	3 Legged	e partition and the same	To Chinanaikanhatti	
40	401.940	3 Legged		To Chinanaikanhatti	
41	403.445	3 Legged	To Babbur	real forces and the control of the c	
42	403.700	3 Legged	To City		
43	403.800	3 Legged	-	To City	
44	403.850	3 Legged	To City	The street factor	
45	404.665	3 Legged	To Avdani Nagar		
46	405.315	3 Legged	-	Akshay Food Park Road	
47	405.725	3 Legged	To Hiriyur City		
48	406.195	4 Legged	To Hiriyur City	To Vishveshwarayya Road	
49	407.505	3 Legged	-	To Kottige Village	
50	408.640	3 Legged	To Somerahalli	afen in hill to a minute in	
51	411.180	4 Legged	To Somerahalli	To Doddaghatta	

Table 0-5 List of Major Junctions

CI NI-	Existing	Type of	Leads To		Remarks
SI. No	km	Intersection	Left	Right	Remarks
1	361.730	3 Legged		To Nayakanahatti (NH-45)	NH-45
2	363.500	4 Legged	Anantapur (SH-48)	To Chitradurga (SH-48)	SH-48
3	404.330	4 Legged	To Bangalore (NH-4)	To Chitradurga	NH-4

SI. No	Existing	Type of	Leads To		Damarka
51. IVO	km	Intersection	Left	Right	Remarks
4	405.850	3 Legged	To Bangalore		100

0.5 Engineering Surveys and Investigations

The general engineering studies conducted along the project stretch include topographic survey, Pavement composition, pavement condition surveys and pavement structural strength.

Topographic Survey

The Topographic Survey for improvement of project road was carried out by GPS, electronic Total Station and Auto level equipments. The detailed survey methodology and specifications followed are as described below.

Initially control points and traverse stations were established using GPS and Total Station equipments. Then using Auto level equipment, Double Tertiary (DT) levelling was carried out to fix temporary benchmarks with respect to GTS Benchmarks available in the area to establish the vertical control to all TBMs and traverse stations. The detailed survey was then carried out and the data was processed for engineering design.

The following specifications were adopted for carrying out the topographic surveys. Traverse points positioning was with accuracy of 1:10,000. All traverse points were marked on ground by punching nails in to surface. TBMs are fixed on BM reference pillars fixed 250 m apart, at top of the kilometre stones, parapets of culverts and on plinth top of the buildings. Double tertiary methods have been used in levelling, taking the sum and average of three wire readings to determine reduced levels.

Cross-sections of roads are taken at an interval of 50 m. Levels were taken at the centre and edges of carriageway, edge of shoulders and levels up to property lines on both sides. Details of drain widths and depths also collected. For all the existing culverts, top levels / road levels and bed levels / sill levels were collected.

Road Inventory

The existing pavement for the entire stretch is of bituminous surface except at major bridge locations. The pavement width is predominantly two lane carriageway with 1.0 to 1.5 m earthen shoulders on either side and 4 lane divided carriageway exists at some built up stretches of the highway and is varies from fair to poor condition.

Pavement Condition

The survey on general pavement condition was primarily a visual exercise undertaken by means of slow drive-over survey, and supplemented with measurements wherever necessary. Visual assessment was carried out at suitable intervals at 200 m and wherever necessary, depending on variations in pavement conditions.

The existing pavement condition based on visual observation varies from good to poor. It has been observed that in most of the stretches heaving / settlements / distress were observed. This may be due to weak subgrade, improper compaction or movement of heavy loaded trucks. Summary of pavement condition is given in Table below.

Table 0-6 Summary of Pavement Condition

SI. No.	Summary	Length (km)	Percentage (%)
1	Good	21.04	40.12
2	Fair	23.75	45.28
3	Poor	7.15	13.63
4	V. Poor	0.50	0.97
	Total	52.44	100

Pavement Structural Strength

Benkelman Beam deflection studies were carried out for evaluating the residual strength of the existing pavement and assessing the strengthening requirements for the existing pavement.

BBD tests have been conducted for every 3 km interval. The pavement deflection has been observed for homogeneous sections based on pavement condition survey.

The summary of Characteristic deflection is given in Table below

Table 0-7 Summary of Characteristic Deflection

SI. No	Chainag NH-1		Characteristic Deflection
ı	From	То	(mm)
1	368.920	369.145	0.93
2	371.960	372.185	1.45
3	374.895	375.120	1.51
4	377.885	378.110	1.47
5	380.860	381.085	1.37
6	382.870	383.095	1.43
7	385.860	386.085	1.69
8	388.840	389.065	1.31
9	391.875	392.100	1.70
10	394.875	395.100	0.61
11	397.875	398.100	0.79
12	400.875	404.355	1.18

Subgrade Investigation

The laboratory investigations of sub-grade indicate that the existing subgrade varies from location to location along the road. The 4 days soaked CBR values for 65 blows of existing sub-grade varies from 9% to 12%.

Material Investigation

Potential sources of soil for construction of embankment and subgrade (for reconstruction / new carriageway) were identified on either side of project stretch.

Borrow area quarries located in 6 locations with average CBR of 10% which is found suitable for construction.

Aggregate quarries were identified in 5 locations out of these 4 locations found suitable for construction.

Hydrological investigations

Hydrological investigations have been carried out for the entire project road. It has been verified that majority of the cross drainage structures are hydrologically adequate to carry the discharges of the river / streams. It has been ascertained from local enquiry and from the National Highways.

0.6 PRELIMINARY DESIGN

Geometric design

The existing alignment largely runs is in plain terrain and the design speed of 80 to 100 kmph has been adopted as per IRC: SP: 84- 2014. Geometric design viz. horizontal and vertical curves are designed as per IRC 38-1988 & IRC: SP: 23 - 1983.

Alignment proposal

After carrying out field investigations and reconnaissance survey of existing alignment, the consultants have been arrived at alignment proposals.

Widening proposals have been prepared based on availability of additional land / existing RoW, horizontal geometrics, study of existing bridges and considering road safety parameter measures. The lengths of adopted type of widening details (as per proposed chainage) are given in Table below.

SI. No Type of widening Design Length km **Eccentric** 1 29.540 2 **Bypass** 22.660 3 Concentric 1.905 Realignment 4 0.600 5 **Toll Plaza** 1.000

Total Length, Km

Table 0-8 Length of adopted type of widening scheme

Proposed Lane widths

The width of basic traffic lane is taken to be 3.5 m. For proposed 4-lane the carriageway width will be 14m width with paved shoulders on both sides. In divided cross-section, the median will be of 5.0 m width in rural areas and of 2.5 m width in urban areas including 0.5 m shy off on both sides. The proposed carriageway details are presented in table below.

Table 0-9 Proposed Lane widths

S. No	Design C	Chainage	Length,	Lane Width Remarks	
5. NO	From	То	Km	Lane Width	Kemarks
1	358+500	389+100	30.600	4 Lane Divided Carriageway	
2	389+100	389+800	0.700	4 Lane Divided Carriageway with Service Road	Yarraballi Built up
3	389+800	414+205	24.405	4 Lane Divided Carriageway	

55.705

Typical Cross Sections

Based on the traffic considerations, geometric standards and existing site conditions, the following typical cross sectional elements are framed for project road.

Table 0-10 Details of Proposed Cross Section

Element	Width (m)	Total Width (m)
4-lane Divided Carriage way Cross Section in Rura	al (C/S Type 1,1A,:	1B,& 1D)
Main Carriageway	2 X 7.00	14
Median	1 x 4.00	4
Kerb Shyness	2 x 0.5	1
Paved shoulder	2 X 1.50	3
Earthen Shoulders	2 X 2.00	4
Space left for Future widening, Earthen drain, Slope Varies	2 x 15.00	30
Space for Services	2 x 2.00	4
Total		60
4-lane Divided Carriage way New Construction for Bypas	s With Service Ro	ad (C/S Type 1C)
Main Carriageway	2 X 7.00	14
Median	1 X 4.00	4
Kerb Shyness	2 x 0.5	1
Paved shoulder	2 X 1.50	3
Earthen Shoulders	2 X 2.00	4
Earthen Shoulders for service roads	2 X 3.00	6
Service Road	2 X 7.00	14
Variable, Space for Services	2 X 7.00	14
Total		60
4-lane Divided Carriage way Cross Section in Urb	oan (C/S Type 2, 2	A & 2B)
Main Carriageway	2 X 7.00	14
Median	1 X 1.50	1.5
Kerb Shyness	2 x 0.5	1
Paved shoulder	2 X 2.00	4
Earthen Shoulders	2 X 2.00	4
Drain/Future Widening	2 x 5.25	10.5
Earthen Shoulders for service roads	2 X1.50	3
Service Road	2 X 7.5	15
Drain	2 X 1.5	3
Space for services	2 x 2.00	4
Total		60
Flyover approach Cross Section with Slip Road in	Rural Areas (C/S	Type 3)
Main Carriageway	2 x 10.50	21
Median	1 x 4.00	4
Median Shy off	2 x 0.50	1
Paved shoulder	2 X 2.00	4
Crash barrier with shyness	2 x 1.00	2
Slip road on both sides	2 x 7.00	14

Element	Width (m)	Total Width (m)
Earthen shoulders	2 x 1.50	3
Divider between slip Road and RE wall	2 x 0.50	1
Slope Varies, Drain / Space for Services	2 x 5.00	10
Total		60
VUP/Flyover approach Cross Section with Slip Ro	oads in Built Up Area (C/S Type 3A)
Main Carriageway	2 x 10.5	21
Median	1 X 1.50	1.5
Median Shy off	2 x 0.5	1
Paved shoulder	2 X 2.0	4
Crash barrier with shyness	2 x 1.0	2
Divider between slip Road and Fly Over Approach	2 x 4.250	8.5
Service road on both sides	2 x 7.50	15
Utility corridor	2 x 2.00	4
Footpath cum drain	2 x 1.50	3
Total		60
VOP Approach Cross Section with S	Slip Road (C/S Type 4)	ov see and intellige
Main Carriageway	2 x 14.00	28
Median	1 x 4.00	4
Median Shy off	2 x 0.5	1
Paved shoulder	2 X 1.50	3
Footpath cum Drain	2 x 1.5	3
Crash Barrier	2 x 1.0	2
Slip road on both sides	2 x 5.5	11
Shy off in Slip Road	2 x 1.0	2
Footpath/Drain in Slip Road	2 x 1.0	2
Space for Services	2 x 2.0	4
Total	the are self to the	60
4-lane Divided Carriage way with Retaining	Wall at Hilly Area (C/S	Type 5)
Main Carriageway	2 x 7.00	14
Median	1 x 4.00	4
Median Shy off	2 x 0.5	1 - 1 - 1
Paved shoulder	2 x 1.50	3
Earthen Shoulders	2 x 2.00	4
Drain, Breast Wall, Catch water Drain and Future		Column 1
widening, Utility corridor etc	2 x 17.00	34
Total		60
Approach of ROB with Slip Ro		
Main Carriageway	2 x 12.00	24
Median	1 x 4.00	4
Median Shy off	2 x 0.5	1
Footpath Shy off	2 x 0.5	1
Footpath	2 x 1.5	3
Footpath Crash barrier	2 x 0.5	1

Element	Width (m)	Total Width (m)
Slip road on both sides	2 x 5.5	11
Divider	2 x 0.5	1
Earthen Shoulder	2 x 1.5	3
Utility Corridor and Drain	2 x 2.0	4
Varies	2 x 3.5	7
Total		60

Pavement Design

New Flexible pavement has been proposed for additional 2-lane & flexible overlay over existing 2-lane road. Rigid pavement is proposed only at toll plaza location.

New Flexible/Rigid Pavement Design

The pavement design basically aims at determining the total thickness of the pavement structure as well as thickness of individual structural components. The following assumptions are considered for the preliminary pavement design. The basic assumptions considered while designing are as follows.

As per IRC: SP: 84-2014 (for 4-Lane)

As per IRC: SP: 84-2014 flexible pavements shall be designed for a minimum design period of 15 years or operation period whichever is more. Stage construction will be permitted subject to the thicknesses of sub-base & base coarses are designed for 15 years & bituminous surface for a minimum of 10 years. Strengthening for the future traffic can be carried out by means of Bituminous Overlay. Rigid pavement shall be designed for a minimum design period of 30 years; stage construction shall not be permitted.

Flexible Pavement

The project road has been divided into three homogeneous sections, design for which are furnished below.

Table 0-11 Cumulative MSA at sections

Description	From D. Ch 3 389+1		From D. Ch 414-	389+150 to +205
ord rescard the restance	Challakere- Hiriyur	Hiriyur- Challakere	Challakere- Hiriyur	Hiriyur- Challakere
Design life of Base and Sub Base (in years) - For Stage Const.	15	15	15	15
Design life of BT layer (in years) - For Stage Const.	10	10	10	10
MSA for Base and Sub Base - For Stage Const.	47.92	22.96	48.90	25.26
MSA for BT layer - For Stage Const.	27.69	13.27	28.26	14.60
Design CBR %	9%	9%	9%	9%

Table 0-12 Flexible pavement composition (in mm) recommended for Main Carriageway

		358+500 to +150	From D. Ch 414-	389+150 to +205
Description	Challakere- Hiriyur	Hiriyur- Challakere	Challakere- Hiriyur	Hiriyur- Challakere
Bituminous Concrete (BC)	40	40	40	40
Dense Bituminous Macadam	95	70	95	70
Wet Mix Macadam	250	250	250	250
Granular Sub Base	200	200	200	200
Sub Total	585	560	585	560
Subgrade of CBR 9% material	500	500	500	500
Total	1085	1060	1085	1060

The pavement composition of paved shoulders has been kept with the same specifications as those of the main carriageway.

Table 0-13: Flexible Pavement Compositions for Service road

Service/Slip Road				
Composition	MSA	Design CBR (%)	Thickness, mm	
ВС	Cluster II	unantial liberal alternative on	40	
DBM	ip gunnin	m & mi acogue	50	
WMM	10		250	
GSB	10	9	200	
Subgrade	orbak Die	re control dvina	500	
Total		o grove inc	1040	

Sub grade of 500 mm thickness of CBR value not less 9% and sub base material of CBR not less than 30% shall be used

Table 0-14 Overlay Thickness (in mm) recommended

NH-150A Existing Km		Design Chainage		Dc	MSA	BM (mm)	Overlay	Design Thickness	
From	То	From	То	, DC			(mm)	BC (mm)	DBM (mm)
368.160	387.410	369+900	389+150	1.404	34.58	145.41	101.787	40	65
387.410	399.560	389+150	401+300	1.12	35.36	107.99	75.593	40	50

Rigid Pavement

The Rigid pavement is proposed at toll plaza location. The design as per the IRC: 58 - 2015 leads to the crust thickness of pavement as given in following table.

Table 0-15 Rigid Pavement Compositions (As per IRC: 58-2015)

For CBR = 9%

Pavement Composition	Thickness in mm		
Design Life (Yrs)	30		
Design CBR (%)	9		
PQC (mm)	280		
DLC (mm)	150		
GSB (mm)	150		
Total	580		

Truck lay-byes

Truck lay-byes are proposed at following locations and new Flexible pavements is proposed at these locations.

SI. No.	Existing km	Design Chainage	Side	Name / Location
1	Bypass	361+200	Both	Challakere Bypass
2	384.435	386+200	Both	Near Golahalli

Rest Area

The location of the Rest area is given below

SI. no	Existing km	Design Chainage	Side	Location Name
1	376.800	378+550	LHS	Near Sanekere
2	Bypass	407+150	RHS	Hiriyur Bypass

Bus Bay with Bus Shelters

There are 18 nos Bus Bay with Bus Shelters and 22 bus shelters are proposed along the project stretch.

Toll Plaza

Rigid Pavement is proposed for the toll plaza location, as it has longer life and can resist the wear and tear caused by the braking forces exerted by heavy vehicles.

One toll plaza is proposed with additional right-of-way, service lanes, toll booths, lighting, weigh-in-motion Weigh Bridge, automatic, semi automatic and manual toll booths, separate lanes for wide bodied vehicles etc. The detail of the proposed toll plaza is given below.

Table 0-16 Proposed Toll Plaza Location

SI. No.		Toll Plaza Location	No of Toll Lanes*		
	Existing Chainage	Design Chainage	Location	LHS	RHS
1	km 395.620	397+400	Near Balenahalli	7+1	7+1

*As per DO No. NHAI/Chairman/Misc./2016 dated 26.12.2016

Proposal for Structures

There is 1 Major bridge, 23 No's, Minor Bridges, 1 No's Minor bridge Cum VUP, 1 No's MNB Cum Flyover, 6 No's Flyovers, 1 No's. Interchange, 5 No's VUP's, 2 No's LVUP's and 1 ROB along with many other cross drainage works are proposed along the project stretch. No's of each type of structure along the project stretch are given in below Table. These structures are proposed for widening, rehabilitation or construction of new structure.

Table 0-17: Proposed Structures on Project Stretch

SI. No.	Type of structure	No's.
1	Major Bridge	01
2	Minor bridges	23
3	MNB Cum VUP	01
4	MNB Cum Flyover	01
5	Flyover	06
6	Interchange	01
7	VUP	05
8	LVUP	02
9	ROB	01
10	Culverts	72
11	Cross Road Pipe Culverts	52
12	Cross Road MNB	1

0.7 Cost Estimation

The cost estimation for the project is extremely important as the viability and implementation of a project depends on the project cost. Therefore, cost estimates have been carried out with due care. Estimation of preliminary cost, a primary pre-requisite for economic and financial evaluation, has been carried out for widening the existing NH stretches to 4-lane carriageway with paved shoulders on both sides including Reconstruction of the existing pavement, strengthening / widening of existing bridge structures, construction of new bridges, rehabilitation and reconstruction / widening of cross drainage structures, longitudinal drains, junction improvements, road furniture, bus bays, truck bays, way side amenities, toll plazas, etc. and is presented in below Table.

Table 0-18 Summary of Cost Estimate for Challakere to Hiriyur section of NH-150A

BILL NO.	BILL NAME		Total Amount in Rs	Total Amount in Crores	Total Amount
	CIVIL CONSTRUCTION COST				11885
1	Site Clearance and Dismantling		63,95,439	0.64	0.07%
2	Earth Work		1,00,96,17,720	100.96	11.74%
3	Granular Sub-Base and Base Courses	1,47,69,27,290	147.69	17.17%	
4A	Flexible Pavement		1,35,21,39,938	135.21	15.72%
4B	Rigid Pavement			1,541	
5	CULVERTS	1,1			
5A	Box Culvert		28,11,71,246	28.12	3.27%
5B	Pipe Culvert	TO WITH THE	9,21,73,672	9.22	1.07%
6	BRIDGES	177714 1.3			
6A	Repair & Rehabilitation of Structures	性的 似的复数	40,14,660	0.40	0.05%
6B	Major Bridges	17 4-20-4	12,02,44,825	12.02	1.40%
6C	Minor Bridges	A STATE OF THE PARTY OF THE PAR	85,46,91,352	85.47	9.94%
6D	VUP	THE RESERVE OF PARTY	71,80,69,153	71.81	8.35%
6E	LVUP	Pip, Tita	15,33,94,187	15.34	1.78%
6F	CUP	9,97,89,921	9.98	1.16%	
6G	ROB		39,46,99,009	39.47	4.59%
6H	Flyover	1,08,53,59,962	108.54	12.62%	
7	Drainage, Protective Works & Other Services	6,91,52,012	6.92	0.80%	
8	Junctions		9,74,53,864	9.75	1.13%
9	Traffic Signs, Road Marking and Other Appurtenances	25,95,95,287	25.96	3.02%	
10	Miscellaneous		41,80,04,850	41.80	4.86%
11	Toll Plaza Construction @ Ch.397+400		10,98,07,651	10.98	1.28%
Α	Total Civil Cost	8,60,27,02,038	860.27		
	Civil Cost per Km (Length of Project H	15.44			
В	Centages over CIVIL cost (15%)		1,29,04,05,306	129.04	
С	TOTAL P	PROJECT COST (A+B)	9,89,31,07,344	989.31	
	Total	Project Cost per Km	17.76		
	PRE CONSTRUCTION ACTIVITY				
	LA, R&R and Social		2,37,65,88,850	237.66	
D	Environment Cost and Utility shifting and other preconstrucion activities		1,81,03,594	1.81	
	Shifting of Electrical Poles/Lines (1	8,60,27,020	8.60		
	Shifting of Water Supply Pipe Lines (1	8,60,27,020	8.60		
	Total cost of preconstruction activities D	2,56,67,46,485	256.67		
E	Total Cap	ital Cost (C+D) =	12,45,98,53,829	1,245.99	
	Total Capital Cost F	22.37			

CONCLUSIONS AND RECOMENDATION

ECONOMIC ANALYSIS

The economic internal rate obtained was return of 18.9% for development of the project road with flexible pavement, which is more than the presented 12% of discounted rate and larger positive value of NPV of net benefits indicate the firm viability of project for the proposed improvement of four laning with paved shoulders and hence recommended for implementation.

FINANCIAL ANALYSIS

It is concluded that the project doesn't provide sufficient returns to equity on commercial BOT (Toll) format with 40 % grant for concession period of 10 years & 15 years. However, if the concession period is increased to 20 years, the project provides sufficient returns to equity at 7.5% Grant. For a concession period of 25 Years, the project doesn't require any grant and provides 16.38% return on equity.

However, the consultants recommend the project on HAM with concession period of 15 years and on EPC mode, as per Policy Matter: Technical ((161/2014) Lt No.11041/218/2007-Admn dated 24.07.2014.

K.M. Alay Mani Kumar

Hational Highways Authority of India